Beaver Creek Corridor Design and Analysi

By: Alex Previte

Overview

- Introduction
- Key concepts
- Model Development
- Design
- Accuracy
- Conclusion





🔶 = Wittenberg

Introduction

 Low head dam : small overflow dam us flow characteristics of a river or stream

Dangers: •Drowning •Alter Ecosystem •User Friendly: Kayaking



Introduction

- HEC-RAS
 - Hydrologic Engineering Centers River Analysis Syste
- Computer program
- Used for analyzing rivers and streams
- Able to compute flow characteristics given certain parameters

Ũ

22

Profile Plot

- Geometry profile
 - Data taken over 300 Ft (horizontally)
 - Conta RG(Co)ta ally)



Cross-Section



- X-axis = Horizontal width of River Station
- Y-axis = Elevation above sea level
- = Bank Station marker
- Contour = shape of creek bed at that station
- Stage Height (Y) = Distance from channel bo mon to surface

Energy Equation



Oby Vo

he

a₁, a₂ = Velocity weighting
coefficients (error in average
velocity)

Development

- Determine and adjust Manning n value
- River bank adjustment
- One low-head dam replaced by a single v-notch drap structure

80

- Hydraulic Jump
 - Subcritical to supercritical

Manning Equation

• Equation:



Manning N

- Similar to
- Shows the
- Higher n means

| <u>8</u> | l | | | <u></u> | |
|---|--------------------------|-------------------|---------------------------------------|----------------------------|----------|
| | ٩., | 8 | ul | ľ. | |
| · · · · · · · · · · · · · · · · · · · | sini <mark>s</mark> tern | i Marian | 00 TRA <u>ST</u> | 945 <u>888</u> 5 | |
| Western Mary and the second | <u>ne d'Arne</u> | <u>and Deed</u> | n <mark>i an_{a Maka}n</mark> | naner erstenning hjörne av | 1999 - A |
| Natur | al streams-minor | streama:(to | pewidth.at.flo | oodstaga.< 100-ft) | |
| , | in Channels | | | nan in provinsi si mining | |
| tene on ofte or deen poole | and BRS mark | <u>=0.030.</u> 00 | ., <u>ДД8а</u> , | <u>a clean etraight</u> | full, |
| 0 — 0.7045 0.7040 <u> </u> | B-mime-nown Bri | ia; linemoia | nkirsinne | ana a | (17) |
| n national and an and an and an an and an | n de gerieden | 19.000 | la pad a kari Marina | hine is second spontations | <i>f</i> |
| il some weeds and stones. | 0.045=== | 0.045= | 6,050 | divane av abo | ve, ti |
| verstages, more inellective | . n n4n | 0.048 | . 0,055 | e. same as abiy | ve, lo |
| 0.080 (-somo os ^{alfa} viki) | ingto signos | | es a portane | .0.045 | |
| | | | | | |
| ~ | 8 ⁸ | | | - ⁷⁸ 8 | 1 |

Froude Number

- Froude Number
- U = Velocity of flow
- g = Acceleration of gravity
- h = Depth of flow relative to the channel bottom

F'r :=

 \mathcal{U}

√ g]hi

Ш

Ш

00

- Unitless



Supercritical and Subcritical Floy

- Is the Froude number > or < than 1
 - Fr>1 = Supercritical
 - Fr<1 = Subcritical
- Supercritical When flow velocity is greater than way velocity

Ш

- Subcritical When flow velocity is less than wave
- Hydraulic Jump Occurs when a flow at high velocity.

Sub or Supercritical Flow





Subcritical Flow III

Supercritige Flow

m

Hydraulic Jumps

With V-notch want to create a hydraulic jump

| • · · · · · · · · · · · · · · · · · · · | | |
|---|-----|------------|
| | | |
| | 7.2 | 57 |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | A0000 (11) |

Designing V-

Ш 'n Ω ø <u>_</u>11 8 œ.

Designing V-notch

- Flow Prior to dam is subcritical
- Fr<1
- Supercritical flow over the dam
- Fr>1
- Not safely passable by kayak or canoe



Designing V-notch

• ()

n 22 _)))

õ,

Hydraulic Jump

• Constant Q (93 cfs)



Stage height at Vnotch is 1.43 ft.





Hydraulic ump?

Hydraulic Jump

- Ratio of height = 1.36
 - is the stage height of the v-notch

The second s

- is the stage height of the following cross-section $\mathbb{U}_{\mathbf{w}}$
- Success!
- Created an undulant jump or rapid which is passable by both canoe and kayak

Ш

80

Hydraulic Jump

• Is the model consistent?

Supercritical to subcritical flow

• Indeed our structure has a Hydraulic jump

45 0.74

<u>226 354.8 80199 0.27846 8 02 cf 75s0 1 224.33 27</u>

Model Accuracy

- Is stage height unreasonably high?
 - Uncharacteristic flow can damage downstream structure
 - Must be consistent with the stage discharge of previous structure

80

• Offset rating curves for comparison

Stage-Discharge Curve

• Pre Modification



CUIVE
 Post Modification
 Stage-Discharge KS11
 Stage-Discharge KS11
 Stage & Stage &

m

Conclusion

- HEC-RAS modeling to create modification
- Model was consistent
- Dam modifications are possible



Acknowledgements

. m

80

- Dr. George
- Dr. Williams

Questions?

